DYNAMIC ANALYSIS OF TRANSMISSION TOWER

A Thesis Submitted In Partial Fulfillment

of the requirement for the award of Degree of

Bachelor of Technology

in

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by

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CERTIFICATE

This is to certify that the project entitled, "**DYNAMIC ANALYSIS OF TRANSMISSION LINE TOWER**" submitted by **BUDHI MAN SHINGDAN** in partial fulfillments for the requirements for the award of Bachelor of Technology Degree in Civil Engineering at National Institute of Technology, Rourkela is an authentic work carried out by him under my supervision and guidance.

To the best of my knowledge, the matter embodied in the thesis has not been submitted to any other University/ Institute for the award of any Degree or Diploma.

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Abstract

Transmission line towers carry heavy electrical transmission conductors at a sufficient and safe height from ground. In addition to their self-weight they have to withstand all forces of nature like strong wind, earthquake and snow load. Therefore transmission line towers should be designed considering both structural and electrical requirements for a safe and economical design.

This paper introduces different types of transmission tower and its configuration as per Indian Standard IS-802. A typical type of transmission line tower carrying 220kV double circuit conductors is modeled and analyzed using STAAD-Pro considering forces like wind load, dead load of the structure and earthquake load as per Indian Standard IS1893:2000(part 1). The transmission tower has height of 40m which includes the ground clearance(h1),maximum sag of the lower most conductors wire(h2),vertical spacing between the conductors wires(h3) and vertical distance of earth wire from the uppermost conductor wire(h3).The earth wire or ground wire is always located at the top of the transmission tower. It has a square base width of 11.5m

The type of transmission tower considered is a tangent tower having no deviation located on a plain landscape with minimal obstacles. It is located at the wind zone 6 with basic wind speed of 55m/s. The wind pressure on the tower depends on the gust response factor **(G_T)** which increases with height.

The transmission tower is situated in the most seismic sensitive region i.e. Zone V where response reduction factor steel frame with concentric braces is of 4 and the damping for steel structures is 2%.

The members are designed for maximum deflection and load for the most critical load combination as per code IS802. The members are also grouped for better fabrication. Steel optimization has been carried out to find the most suitable and economical section for the design.

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Introduction

In every country, the need of electric power consumption has continued to increase, the rate of demand being greater in the developing countries. Transmission tower lines are one of most important life-line structures. Transmission towers are necessary for the purpose of supplying electricity to various regions of the nation .This has led to the increase in the building of power stations and consequent increase in power transmission lines from the generating stations to the different corners where it's needed. Interconnections between systems are also increasing to enhance reliability and economy. Transmission line should be stable and carefully designed so that they do not fail during natural disaster. It should also conform to the national and international standard. In the planning and design of a transmission line, a number of requirements have to be met from both structural and electrical point of view. From the electrical point of view, the most important requirement is insulation and safe clearances of the power carrying conductors from the ground. The cross-section of conductors, the spacing between conductors, and the location of ground wires with respect to the conductors will decide the design of towers and foundations.

The major components of a transmission line consist of the conductors, ground wires, insulation, towers and foundations. Most of the time transmission lines are designed for wind and ice in the transverse direction. However, the Indian Sub-continent is prone to moderate to severe earthquakes seismic loads may be important because the transmission line towers and the cables may be subjected to higher force and stressed during ground motion. However, the major concern of the transmission line during high earthquakes may be that the large displacements do not causes the cables to touch each other or any surrounding objects, causing power failure and accidents. Therefore, earthquake forces may be important in design in high earthquake zones of the country. In this project Seismic behavior of transmission line is determined from the dynamics analysis of the tower and the cable subjected to earthquake ground motion.

Types of towers *clause 6 IS 802 (Part 1/Set 1) : 1995*

The selection of the most suitable types of tower for transmission lines depends on the actual terrain through which the line traverses. Experience has, however, shown that any combination of the following types of towers are generally suitable for most of the lines:

Suspension towers

Suspension towers are used primarily on tangents but often are designed to withstand angles in the line up to two degrees or higher in addition to the wind, ice, and broken-conductor loads. If the transmission line traverses relatively flat, Featureless terrain, 90 percent of the line may be composed of this type of tower.

Tension towers

As they must resist a transverse load from the components of the line tension induced by this angle, in addition to the usual wind, ice and broken conductor loads, they are necessarily heavier than suspension towers

Table 1. Type of tower

A) Small angle towers (0” to 15’) with tension string	**Deviation of 0” to 15”.**
B) Medium angle towers (0” to 30”) with tension string	Deviation of 0” to 30”.
C) Large angle towers (30” to 60”) with tension string	Deviation of 30” to 60”.
D) Dead-end towers with tension string	To be used as dead-end (terminal) tower or Anchor tower.
E) Large angle and dead-end towers with tension string	To be used for line deviation from 30” to 60” or for dead-ends.

IS: 802(Part I)-1995 recommends the above classification

Following are the different parts of a transmission tower,

1) Peak of the tower
2) Cage or hamper of the tower, that supports the cross arm.
3) Cross arm for carrying conductors.
4) Tower body, includes bracing
5) Legs of the tower

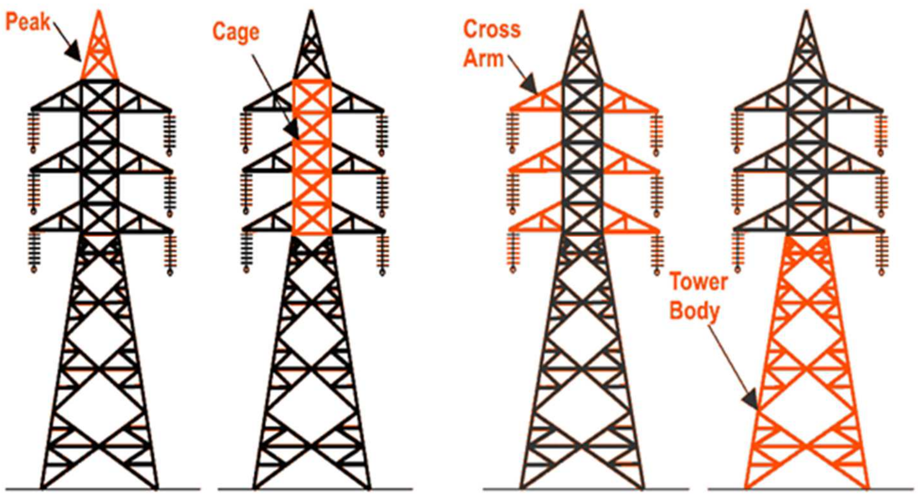


Figure 1. Parts of transmission tower

Tower configuration

The selection of a best outline and the bracing system patterns contributes to a great extent in developing an economical and safer design of a transmission line tower.

For a particular tower configuration selected, the outline decided shall satisfy both electrical and structural requirements at the same time the configuration should be economical.

The square type broad base tower are the most economical and most commonly used in India.

The tower outline diagram comprises

(a) Tower height considered from ground level

(b) Length of the cross-arm, and phase spacing

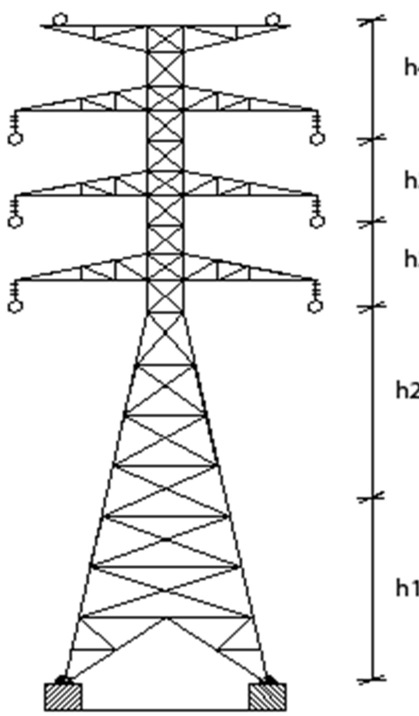
(c) Tower width at (i) base (ii) top hamper

(d) Bracing pattern considered.

Determination of tower height

The factors that govern the height of the tower are:

1. Minimum permissible ground clearance (h1)
2. Maximum sag of the lowermost conductor wires (h2)
3. Vertical spacing between conductors (h3)
4. Vertical distance between ground wire and top conductor (h4)



The total height of the tower is
$H = h_1 + h_2 + 2h_3 + h_4$

Figure 2. Tower height

Minimum permissible ground clearance (h_1)

For safety, power carrying conductors along the path of the transmission line should maintain minimal clearance to ground, highways roads, rivers, railways tracks, telecommunication lines, other power lines, etc.., as according to the Indian Electricity Rules, or as per Indian standards.

Rule 77(4) of the Indian Electricity Rules, 1956, suggests the following minimum clearances above ground of the lowest point of the conductor:

The clearance above ground shall not be less than the following figures plus the height of the nearby obstacles.

The values of minimum ground clearance for the various voltages ranging from 66kV to 400 kV, are:

66kV – 5.47m
132kV – 6.10m
220kV – 7.01m
400kV – 8.84m

Maximum sag of the lowermost conductor (h_2).

The power carrying conductors sags due to its self-weight and the sag is maximum when the temperature is maximum and when there is no wind condition. The maximum sag occurs at the mid-section between the two towers in open country.

$$\max sag = \frac{q\,W\,l^2}{8f}$$

q = loading factor $\sqrt{\frac{w^2+p^2}{w^2}}$

W=weight of the conductor/m/cm^2

l =span length in meters.

f =working stress of conductor.

Spacing of conductors (h_3)

They should be adequate vertical space between the conductors so that they do not touch each other during dynamic loads such as during high earthquake and high wind.

The following is the vertical clearances generally allowed at the mid of the span between the conductors.

Table 2 Vertical clearance permissible of the middle of span

Span(m)	Vertical clearance permissible of the middle of span.(mm)
200	4,000
300	5,500
400	7,000
500	8,500

Vertical clearance between ground wire and top conductor (h_4)

The vertical spacing between conductors and the earth wires is governed by shield angle, i.e. angle which the line joining the ground wire and the outermost conductor makes with the vertical which is required to protect the conductors wires and the transmission tower form the direct lighting strokes. Generally the shield angle varies from 25^o to 30^o which depend on the overall configuration of the transmission tower and the amount of voltage the transmission line carries.

Table 3. Vertical and horizontal spacing between conductors

Type of tower	Vertical spacing between conductors (mm)	horizontal spacing between conductors (mm)
1. 66 kV: Single circuit		
A(0-2°)	1,030	4,040
B(2-30°)	1,030	4,270
C(30-60°)	1,220	4,880
2. 66 kV: Double circuit		
A(0-2°)	2,170	4,270
B(2-30°)	2,060	4,880
C(30-60°)	2,440	6,000
3. 132 kV: Single circuit		
A(0-2°)	4,200	7,140
B(2-15°)	4,200	6,290
C(15-30°)	4,200	7,150
D(30-60°)	4,200	8,820
4. 132 kV: Double circuit		
A(0-2°)	3,965	7,020
B(2-15°)	3,965	7,320
C(15-30°)	3,965	7,320
D(30-60°)	4,270	8,540
5. 220 kV: Single circuit		
A(0-2°)	5,200	8,500
B(2-15°)	5,250	10,500
C(15-30°)	6,700	12,600
D(30-60°)	7,800	14,000
6. 220 kV: double circuit		
A(0-2°)	5,200	9,900
B(2-15°)	5,200	10,100
C(15-30°)	5,200	10,500
D(30-60°)	6,750	12,600
7. 400 kV: Single circuit	horizontal configuration	
A(0-2°)	7,800	12,760
B(2-15°)	7,800	12,640
C(15-30°)	7,800	14,000
D(30-60°)	8,100	16,200

Determination of base width

The base width of the transmission tower is the center to center distance between the adjacent corner legs of the tower at ground level. The base width of the transmission tower plays important role in its stability. Higher the base width higher will be the stability. But the cost will increase with the increase in the base width. Therefore base width should be selected such that it gives the high stability at minimum cost.

Generally for most towers the ratio of base width to total tower height is about one-third to one-sixth from large-angle towers to tangent towers.

Ryle has given the following formula for a determination of the economic base width:

$$B = 0.42\sqrt{M} \text{ or } 0.013\sqrt{m}$$

Where B = Base width in meters,

m= Overturning moment about the ground level in tonne-meters, and

M= Overturning moment about the ground level in kg.meters.

Determination of top hamper width

The top hamper is the part of the tower which supports the cross arm for the conductors wire. It is the width of the tower at the level of the lower Cross-arm.

Table 4.Tower configuration

Type of tower	Width at base (mm)	Top hamper (mm)	Total height (mm)	Vertical spacing between towers (mm)	Horizontal distance between towers (mm)
i) 66kV:Single circuit					
A($0-2^0$)	1,675	760	15,910	1,030	4,040
B(2^0-30^0)	2,590	915	15,425	1,030	4,270
C(30^0-60)	3,050	1,220	16,240	1,220	4,880
(ii)132kV:Double circuit					
A($0-2^0$)	4,050	1,250	26,230	3,965	7,020
B(0^0-15^0)	5,490	1,540	26,545	3,965	7,320
C(15^0-30^0)	4,880	1,665	26,545	3,965	7,320
D(30^0-60^0/D.E)	6,400	1,840	28,060	4,270	8,540

(*iii*)132kV:Single circuit					
A($0 - 2^0$)	3,920	1,300	23,140	42,000	7,140
B($0^0 - 15^0$)	4,224	1,400	22,060	42,000	6,290
C($15^0 - 30^0$)	4,828	1,600	22,685	42,000	7,150
D($30^0 - 60^0$/D.E)	6,135	2,000	24,060	42,000	8,820
(iv) 220kV: Double circuit					
A($0 - 2^0$)	7,000	1,500	28,555	5,200	8,500
B($2^0 - 30^0$)	8,900	1,470	29,080	5,250	1,0500
C($30^0 - 60$)	10,344	2,000	31,680	6,700	1,2600
(v) 220kV: Single circuit					
A($0 - 2^0$)	4,000	1,500	31,650	5200	9,900
B($2^0 - 30^0$)	4,800	1,700	31,300	5200	10,100
C($30^0 - 60$)	8,800	2,000	29,900	5200	9,700
(vi) 400kV: Single circuit					
A($0 - 2^0$)	5,000	2,000	34,100	7,800	12,760
B($0^0 - 15^0$)	5,700	2,000	33,100	7,800	12,640
C($15^0 - 30^0$)	6,900	2,200	33,010	7,800	14,000
D($30^0 - 60^0$/D.E)	7,500	2,400	33,410	8,100	16,200

Bracing systems

Bracing makes the structure stable and rigid. Following are the most adopted bracing system for transmission line towers. Bracing patterns should be selected considering both economic and structural stability.

Single web system

It consists either diagonals or struts. This bracing system is mostly used for small-based towers, in cross-arm. This type of bracing system is usually use for 66 kV single circuit towers, and has little application for wide-based towers of higher voltages

Double web or Warren system

This bracing system is of diagonal cross bracings. Shear is equally carried by the two diagonals, one in compression and the other in tension but both the diagonals are designed for compression and tension so that it will not fail when the load reverses. The diagonal braces are connected at their cross points. Critical length is approximately half that of a corresponding single web system as the shear preface is carried by two members. This system of bracing is economically adopted for both high and small towers.

Pratt system

This system also has diagonal cross bracings and, in addition, it has horizontal struts. These struts are subjected to compression and the shear is taken entirely by one diagonal in tension, the other diagonal acting like a redundant member.

It is economical to use the Pratt bracings for the bottom and Warren bracings at the upper heights.

Portal system

The diagonals are designed for carrying both tension and compression forces therefore its provides stiffness than the other system. For better performance both warren and Pratt system can be used with portal system.

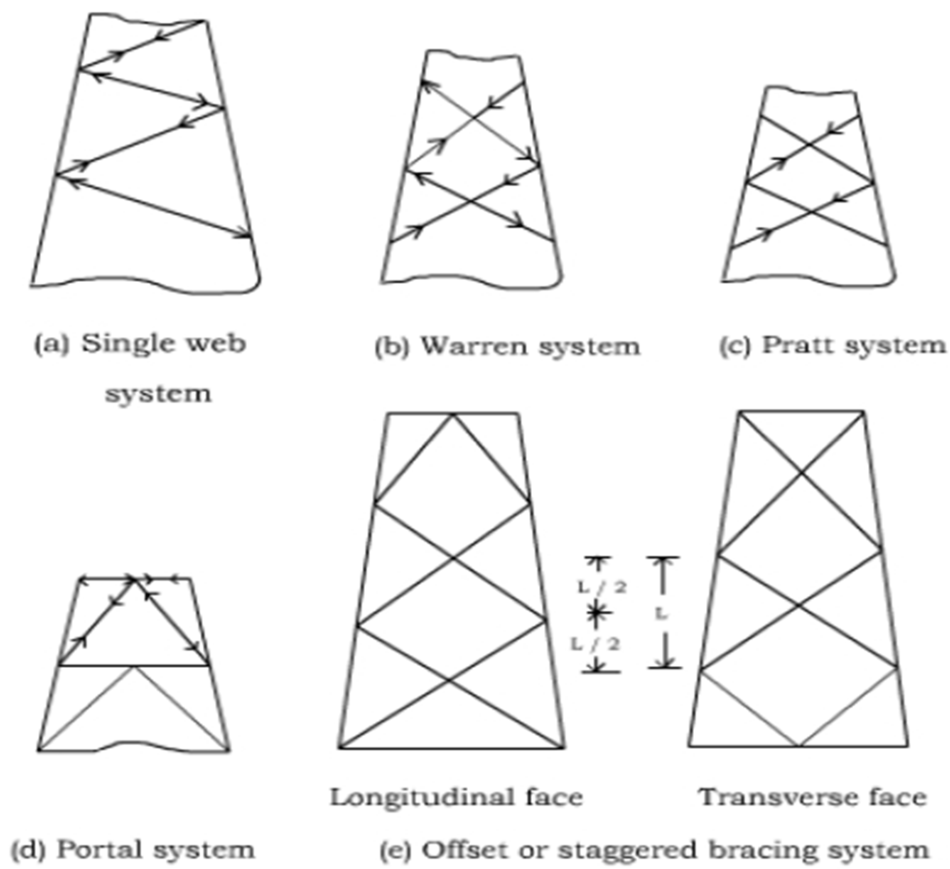


Figure 3 Bracing patterns

Tower configuration considered in this project.

- Type of tower: A-type with 0^0 deviation located in a plain country.

  Tangent suspension tower with double circuit carrying 220kV power.

- **Height of the tower**.

  Minimum ground clearance plus max.sag of the lowermost wire= 25m

  Vertical spacing between the conductors=5m

  Vertical spacing between conductor and the ground wire=5m

Total height of the tower=40m

- Length of the insulator string with dish size 225mmX145mm,

  28 no. of disk, length of string=**2640mm.**

- Base width of the tower=$\frac{1}{3}$ to $\frac{1}{6}$ of the total height.

  =11.5m (square base)

- Width of the top hamper =**2m**

- Length of the cross arm=7.57from the edge of the hamper.

  =7.57+2+7.57

  =17m

- Type of bracing = diamond bracing system.
- Span of tower is **150m** between two towers.
- Conductor Material: ACSR, (Aluminium Conductor Steel Reinforced)

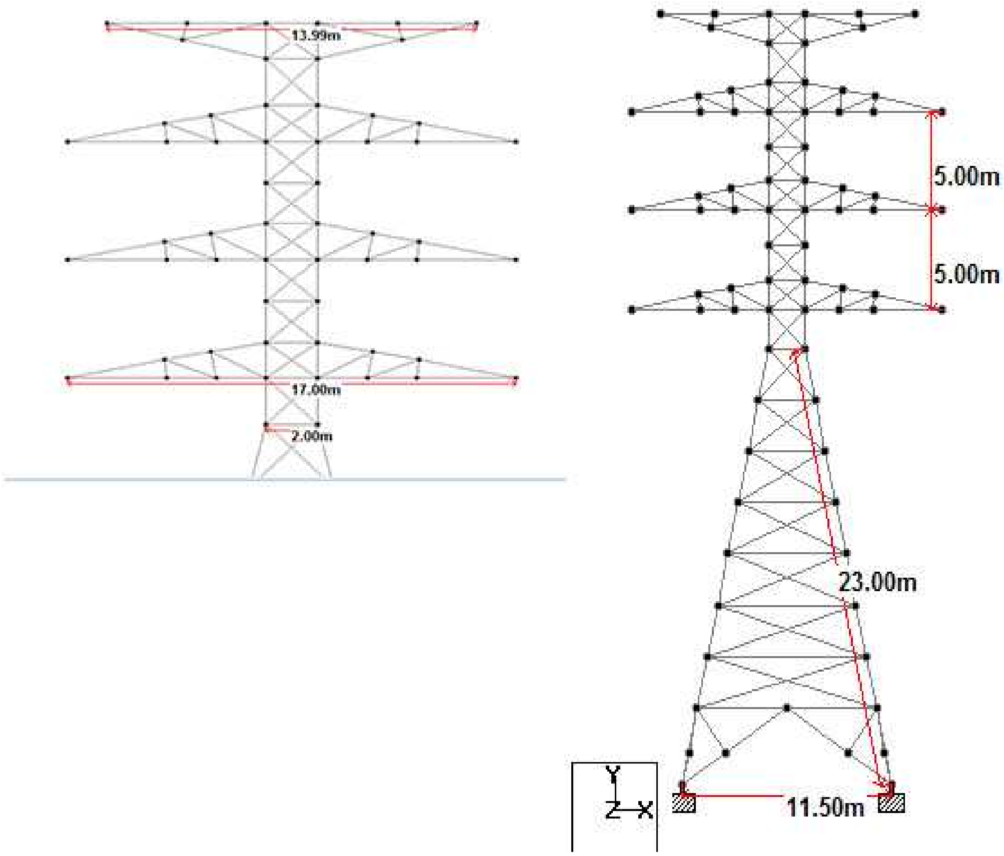


Figure 4. Tower configuration considered

LOADS

The load acting on the towers are

1. Dead load. Self-weight of the tower and the conductors and wires.
2. Wind load calculated as per IS 802 (part1/sec 1): 1995
3. Earthquake load as per IS1893(part 1):2000

Wind load, *clause 8 IS 802 (Part l/Set 1) : 1995*

Figure 5 shows basic wind speed map of India as applicable at 10 m height above mean ground Level for the six wind zones of the country. Basic wind speed 'V_b' is based on peak gust velocity averaged over a short time interval of about 3 seconds, corresponds to mean heights above ground level in an open terrain (Category 2) and have been worked out for a 50 years return period [Refer IS 875 (Part 3) : 1987]

India is divided into 6 wind zones. Basic wind speeds for the six wind zones (see Fig. 5) are,

Table 5 basic wind speed

Wind Zone	*Basic Wind Speed, V_b m/s*
1	33
2	39
3	44
4	47
5	50
6	55

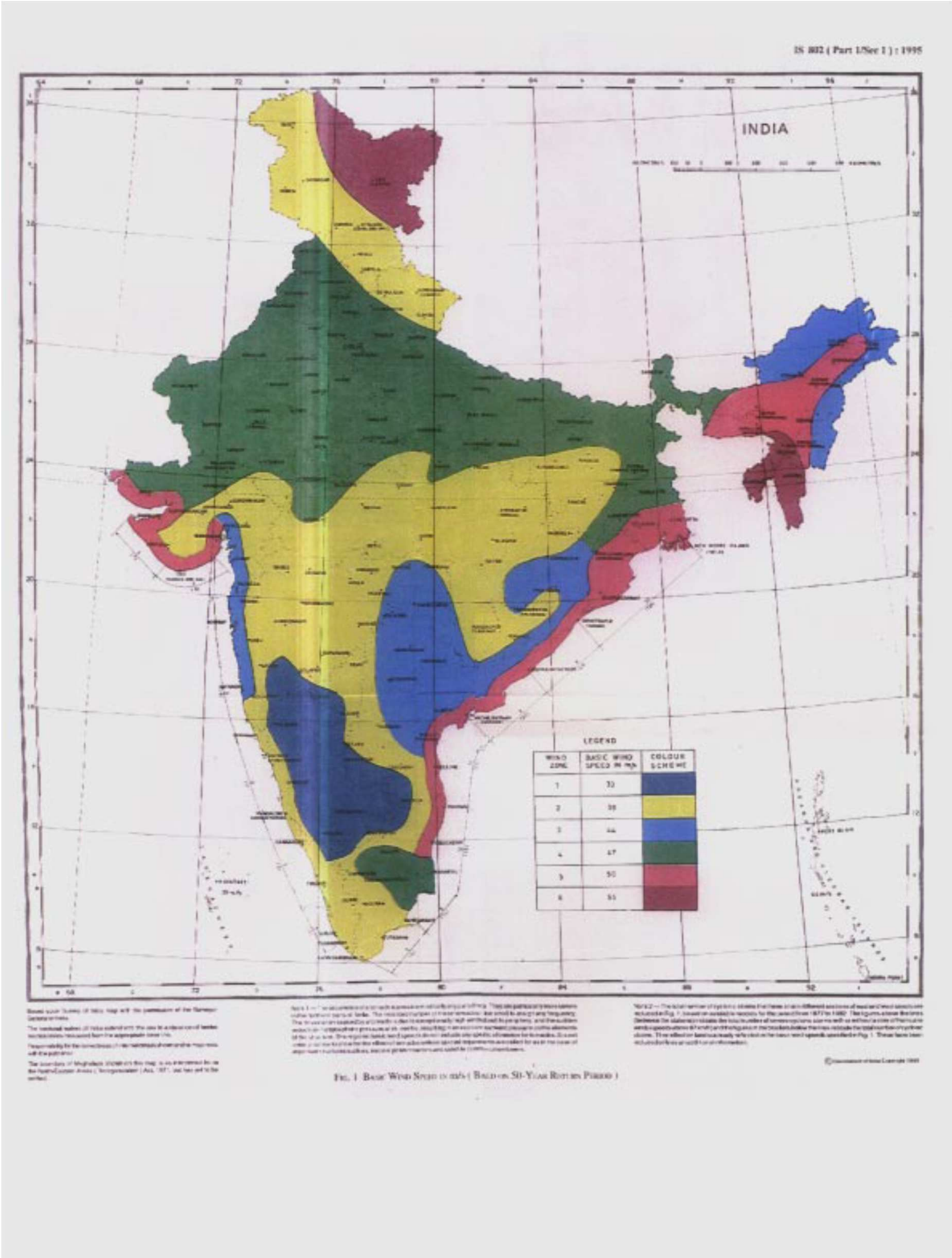


Figure 5. Basic wind speed map in m/s(base on 50years return period)

Design Wind speed, V_d

To get the design wind speed the basic wind speed is modified to include the following effects:

a) Risk coefficient, K_1; and

b) Terrain roughness coefficient, K_2.

Hence it may be expressed as follows:

$$V_d = V_R \times K_1 \times K_2$$

Risk Coefficient, K_1

Table 6. The values of risk coefficients K_1 for different wind zones for the three reliability levels.

Reliability Level	Coefficient K_1 for Wind Zones					
	1	2	3	4	5	6
(1)	(2)	(3)	(4)	(5)	(6)	(7)
1	1·00	1·00	1·00	1·00	1·00	1·00
2	1·08	1·10	1·11	1·12	1·13	1·14
3	1·17	1·22	1·25	1·27	1·28	1·30

Terrain Roughness Coefficient, K_2

Table 7. The values of coefficient K, for the three categories of terrain roughness corresponding to 10 minutes averaged wind speed.

Terrain Category	1	2	3
Coefficient, K_2	1·08	1·00	0·85

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Terrain Category	1	2	3
Coefficient, K_2	1·08	1·00	0·85

Terrain categories

a) **Category 1** - Exposed open terrain land scape with few or no obstacles.

This category includes open seacoasts, deserts and flat treeless, deserts and flat treeless plains.

b) Category 2 - Open terrain with scattered obstructions having height generally between 1.5 m to 10 m.

This category includes normal country lines with very few obstacles.

c) Category 3 - Terrain with number of closely spaced obstructions.

This category includes urban areas and forest areas.

Design Wind Pressure, *Pd*

The design wind pressure which is distributed along the height of the towers, conductors and insulators shall be determined by the following expression:

$$P_d = 0.6 V_d^2$$

P_d=design wind pressure in n/m^2

V_d=design wind speed in m/s

Table 8. The design wind pressures P_d for the three reliability level and pertaining to six wind zones and the three terrain categories have

Reliability Level	Terrain Category	Design Wind Pressure P_d for Wind Zones					
		1	2	3	4	5	6
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)
1	1	403	563	717	818	925	1 120
	2	346	483	614	701	793	960
	3	250	349	444	506	573	694
2	1	470	681	883	1 030	1 180	1 460
	2	403	584	757	879	1 010	1 250
	3	291	422	547	635	732	901
3	1	552	838	1 120	1 320	1 520	1 890
	2	473	718	960	1 130	1 300	1 620
	3	342	519	694	817	939	1 170

Wind Load on Tower

The whole tower is divided into different panels having different height *'h'* to determine the wind load on tower. The wind load is distributed along these height of the tower according to gust response factor. These panels should usually be taken between the intersections of the legs and bracings. For a tower of square base cross-section, the resultant wind load, for wind normal direction on face of tower, on a panel height 'h' applied is;

$$\mathbf{F_{wt} = P_d \times C_{dt} \times A_e \times G_T}$$

Where,

P_d = design wind pressure, in N/m":

C_{dt} = drag coefficient for panel towards which the wind is blowing.
Values of C_{dt} depends on the solidity ratios.
Solidity ratio is equal to the effective area (projected area of all the individual elements) of a frame towards the wind direction divided by the area enclosed by the boundary of the frame normal to the wind direction;

A_e =total net surface area of the legs, bracings, cross arms and secondary members of the panel projected normal to the face in m'. (The projections of the bracing elements of the adjacent faces and of the plan-and-hip bracing bars may be neglected while determining the projected surface of a face);
G_T =gust response factor, peculiar to the ground roughness and depends on the height above ground. Values of GT for the three terrain categories are given in Table.

Table 5 Drag Coefficient, C_{dt} for Tower

(*Clause* 9.1)

Solidity Ratio (1)	Drag Coefficient C_{dt} (2)
Up to 0·05	3·6
0·1	3·4
0·2	2·9
0·3	2·5
0·4	2·2
0·5 and above	2·0

Table 6 Gust Response Factor for Towers (G_T) and for Insulators (G_I)

(*Clauses* 9.1 *and* 9.3)

Height Above Ground	Values of G_T and G_I for Terrain Categories		
m	1	2	3
(1)	(2)	(3)	(4)
Up to 10	1·70	1·92	2·55
20	1·85	2·20	2·82
30	1·96	2·30	2·98
40	2·07	2·40	3·12
50	2·13	2·48	3·24
60	2·20	2·55	3·34
70	2·26	2·63	3·46
80	2·31	2·69	3·58

NOTE — Intermediate values may be linearly interpolated.

Table 9. Drag coefficient for tower

The projected area= *projected area of all the individual element/ enclosed area by the boundary of the frame normal to the wind direction*.

Calculation of exposure area.

Area of tower body.

(11.5+2)/2*22.5=**151.87m²**

Area of cage.

17*2=**34m²**

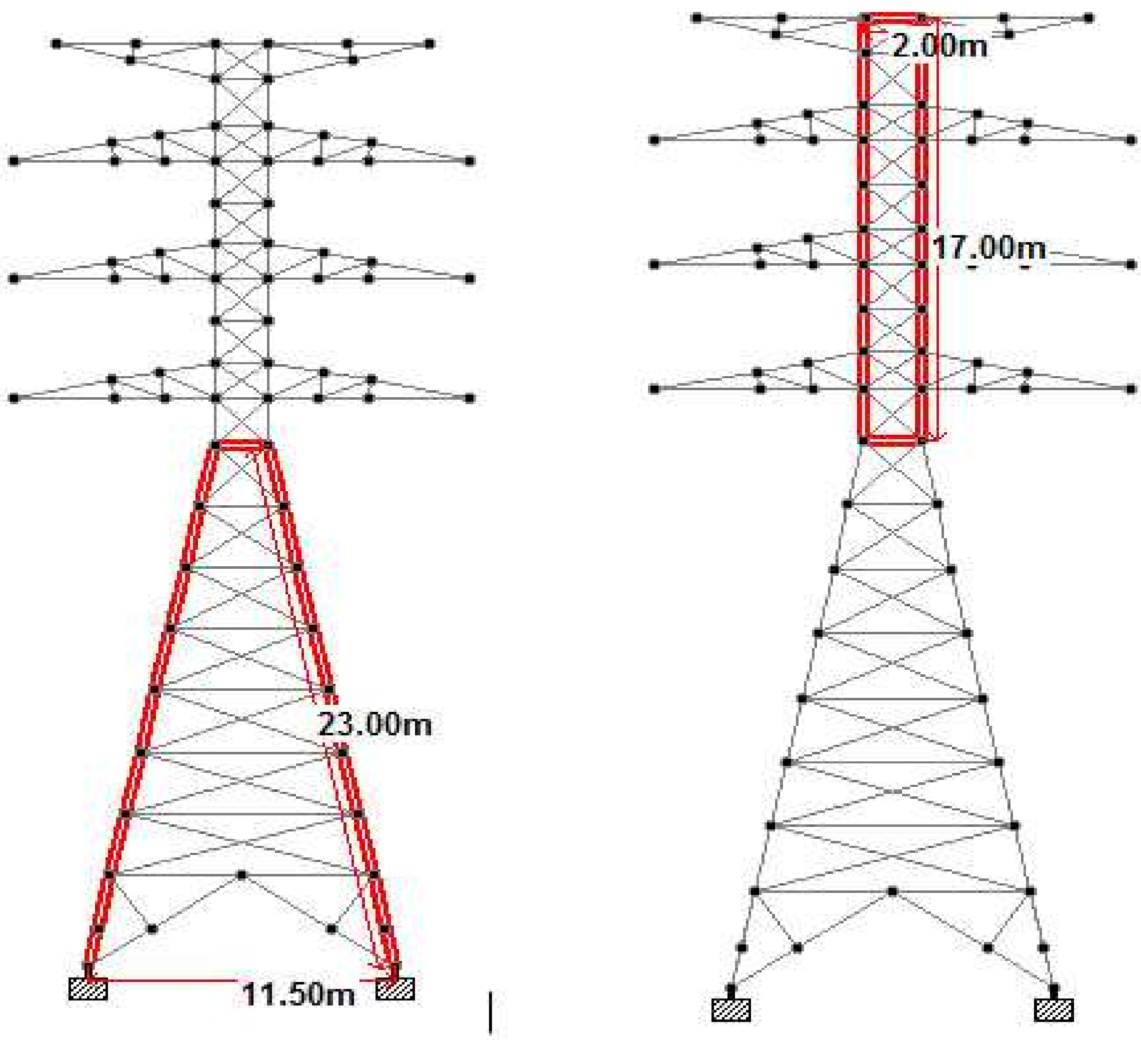


Figure 6. Calculation of projection area of tower body

Area of cross arm

½*7.57*1.5=5.67

5.67*6=34m²

Area of insulator cross arm

½*6.08*1.5=4.56

4.56*2=9.12 m²

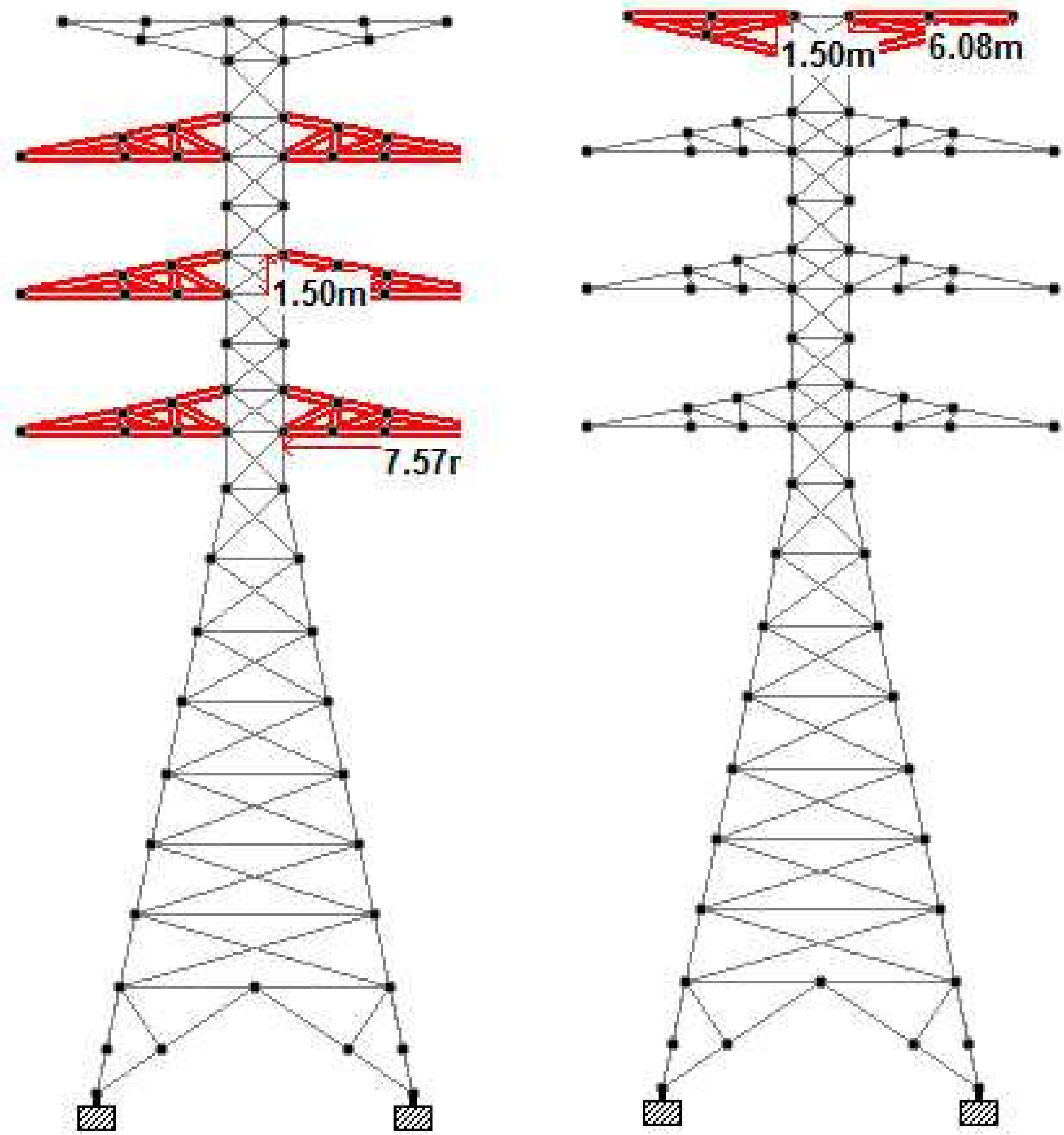


Figure 7. Calculation of projection area of cross arm

Total enclosed area = 151.87+34+34.06+5+9.12=229m2=**230m^2**

Table 10. Total projected area

Section	width	length, m	Area (m^2)
ISA150X150X15	0.15	46	6.9
ISA110X110X10	0.11	34.24	3.7664
ISA100X100X10	0.1	93.16	9.316
ISA90X90X10	0.09	67.06	6.0354
ISA75X75X10	0.075	29.08	2.181
ISA70X70X10	0.07	24.28	1.6996
ISA60X60X10	0.06	184.94	11.0964
ISA55X55X10	0.055	51.66	2.8413
Total projected area			**43.8361**

Therefore the projected area= 43.83/230*100=19.05=**20%**
Wind pressure shall be calculated on **1.5** times the projected are of members on windward face.
1.5x20%=**35%**

Determination of wind pressure, P_d

Basic wind speed (V_b)=55m/s ---------------------zone 6

Meteorological reference wind speed,

$$\boxed{V_R = V_b / K_o}$$

K_o=1.375

Therefore V_R=55/1.375=**40m/s**

Design wind speed,V_d

$$\boxed{V_d = V_R x K_1 x k_2}$$

K_1=risk coefficient=1.3-----------------------for zone 6 with reliability level 3

K_2=terrain roughness coefficient

K_2=1.08---for terrain category 1

V_d=40x1.3x1.08=**56.16m/sec**

Design wind pressure,

$$\boxed{P_d = 0.6 x V_d^2}$$

P_d =0.6x56.16^2 =**1890** N/m^2

Wind Load on Conductor and Ground wire

The following expression gives the wind load that each conductor and ground wire carries:

$$F_{wc} = C_{dc} \times P_d \times L \times d \times G_c$$

Where,

Pd = design wind pressure, in N/m^2;

C_{dc} = drag coefficient, taken as 1.0 for conductor and 1.2 for ground wire;

L = wind span, in meters;

d = diameter of cable, in meters; and

G_c = gust response factor, which take account the turbulence of the wind at different height.

Table 11. Value of G_c for the terrain categories and the average height of the conductor/ground wire

Terrain Category (1)		Height Above Ground, m (2)	Values of G_c for Ruling Span of, in m: Up to 200 (3)	300 (4)	400 (5)	500 (6)	600 (7)	700 (8)	800 and above (9)
1	Up to	10	1·70	1·65	1·60	1·56	1·53	1·50	1·47
		20	1·90	1·87	1·83	1·79	1·75	1·70	1·66
		40	2·10	2·04	2·00	1·95	1·90	1·85	1·80
		60	2·24	2·18	2·12	2·07	2·02	1·96	1·90
		80	2·35	2·25	2·18	2·13	2·10	2·06	2·03
2	Up to	10	1·83	1·78	1·73	1·69	1·65	1.60	1·55
		20	2·12	2·04	1·95	1.88	1·84	1.80	1·80
		40	2·34	2·27	2·20	2·13	2·08	2·05	2·02
		60	2·55	2·46	2·37	2·28	2·23	2·20	2·17
		80	2·69	2·56	2·48	2·41	2·36	2·32	2·28
3	Up to	10	2·05	1·98	1·93	1·88	1·83	1·77	1·73
		20	2·44	2·35	2·25	2·15	2·10	2·06	2·03
		40	2·76	2·67	2·58	2·49	2·42	2·38	2·34
		60	2·97	2·87	2·77	2·67	2·60	2·56	2·52
		80	3·19	3·04	2·93	2·85	2·78	2·73	2·69

NOTE — Intermediate values may be linearly interpolated.

L=150m

P_d =1890 N/m^2

C_{dc}=1

Calculation of wind load on wires

1) Wind Load on lowest Conductor (G_c =1.9) ht=25m

=1890x1x150x0.016x1.9=9072 N=**9.072 kN**

2) Wind Load on mid Conductor (G_c =2) as ht=30m

=1890x1x150x0.016x2= 9072 N=**9.072 kN**

1) Wind Load on top Conductor (G_c =2.05) as ht=35m

   1890x1x150x0.016x2.05=9298.8 N=**9.29 kN**

2) Wind Load on insulator wire.(C_{dc}=1.2,d=20,ht=40, G_c=2.07)

   1890x1.2x150x0.02x2.07=**14.2 kN**

Transverse load

The transverse load consists of loads at the points of conductor and ground wire support in a direction parallel to the longitudinal axis of the cross arms, plus a load distributed over the transverse face of the structure due to wind on the tower

Thus total transvers load

= F_{wt} + F_{wc}+ F_{wi}

where

F_{wi} and F_{wc} are to be applied on all conductors/ground wire points and

F_{wt} to be applied on tower at ground wire peak and cross arm levels and at any one convenient level between bottom cross arm and ground level for normal tower. In case of tower with extensions, one more application level shall be taken at top end of extension.

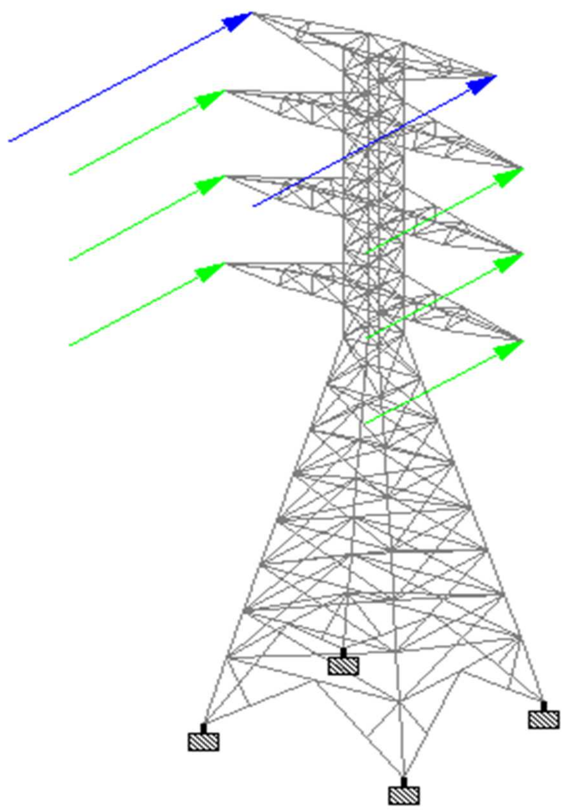


Figure 8. Transverse load

Vertical load

Vertical load is applied to the ends of the cross- arms and on the ground wire peak and consists of the following vertical downward

Components:

1. Weight of bare or ice-covered conductor, as specified, over the governing weight span.
2. Weight of insulators, hardware, etc., covered with ice, if applicable.
3. Arbitrary load to provide for the weight of a man with tools.

Dead load of the wire and insulator disk=7000 N =7kN

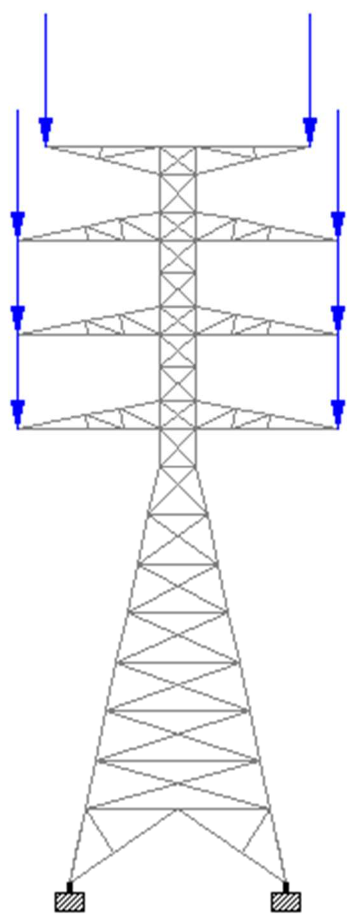


Figure 9. Vertical load

Earthquake load

Steel frames shall be so designed and detailed as to give them adequate strength, stability and ductility to resist severe earthquakes in all zones classified in IS 1893 (Part 1) without collapse. Frames, which form a part of the gravity load resisting system but are not intended to resist the lateral earthquake loads, need not satisfy the requirements of this section, provided they can accommodate the resulting deformation.

Following figure show the seismic parameters considered for defining the earthquake load case in STAADPro

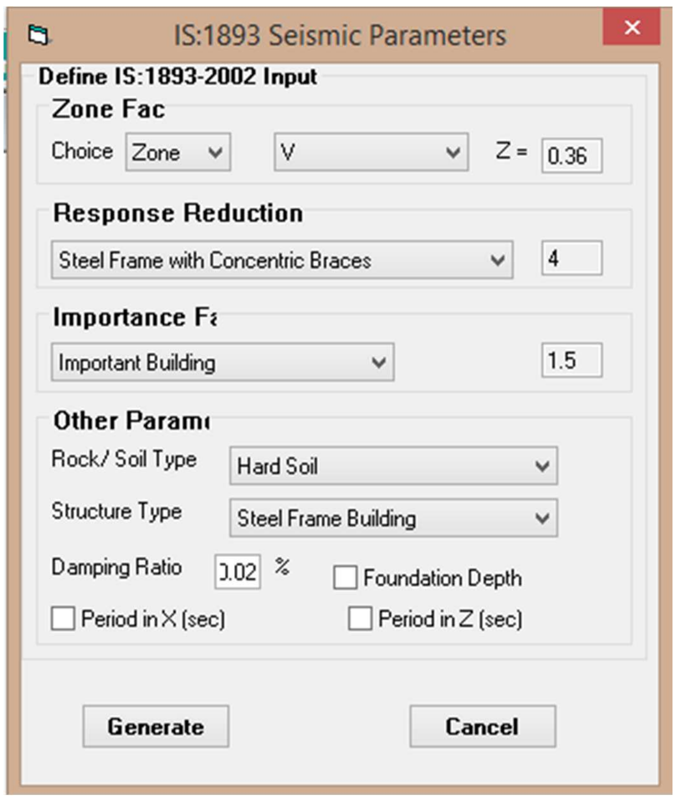


Figure 10. Earthquake load parameters

STAADPRO ANALYSIS

GEOMETRY

Total of 665 no of beam angle section ranging from ISA150x150x18 to ISA45x45x10 are modeled using coordinated system.

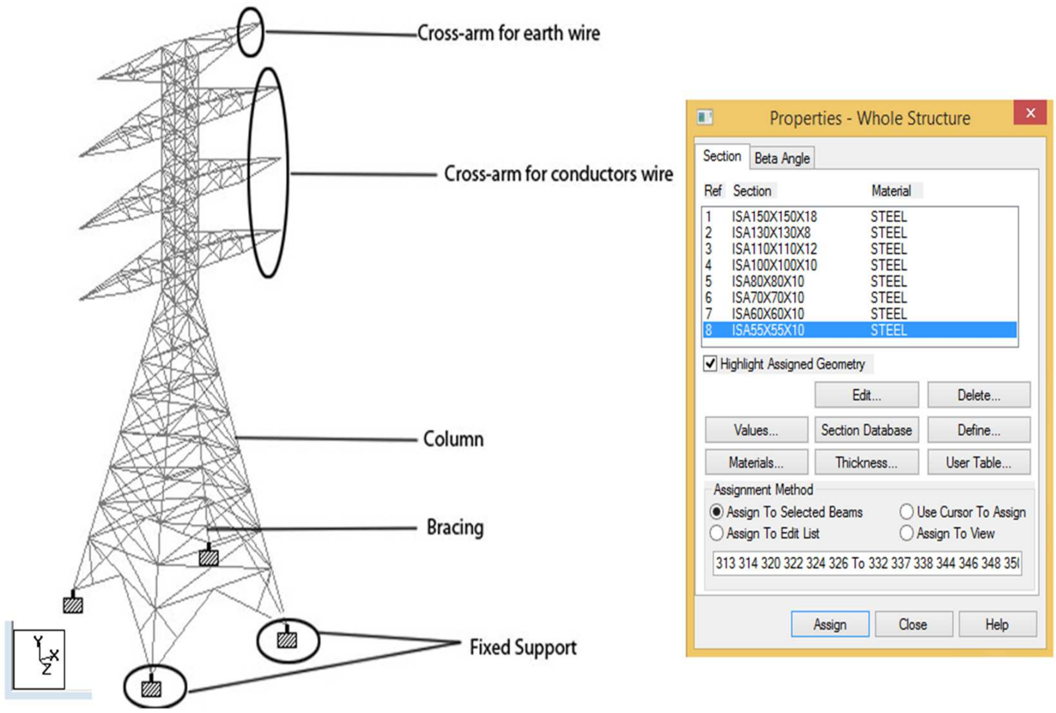


Figure 11. Tower model in STAADPro

The column and the bracing at the lower part of the transmission tower has been assigned with higher angle section than that of upper part of the tower. This is due to the fact that the lower members have to withstand more loads than that at upper part.

Load case details

1) Earthquake load

2) Dead load

3) Wind load

Load combination

Auto load combination as per Indian standard.

4) 1.7xload (2)

5) 1.7xload (2)
 1.7xload (1)

6) 1.7xload (2)
 -1.7xload (1)

7)1.3xload (2)
 1.3xload (1)

8) 1.3xload (2)
 -1.3xload (1)

9)1.7xload (2)
 1.7xload (3)

10 1.7xload (2)
 -1.7xload (3)

11) 1.3xload (2)
 1.3xload (3)

12) 1.3xload (2)
 -1.3xload (3)

Results

Displacement

All | Summary

	Node	L/C	Horizontal X mm	Vertical Y mm	Horizontal Z mm	Resultant mm	Rotational rX rad	rY rad	rZ rad
Max X	47	9 GENERATE	5.593	-2.907	-0.729	6.345	0.001	-0.002	-0.001
Min X	52	9 GENERATE	-5.593	-2.907	-0.728	6.345	0.001	0.002	0.001
Max Y	88	10 GENERAT	0.109	16.254	-299.793	300.233	-0.019	-0.001	-0.000
Min Y	220	10 GENERAT	0.173	-21.741	-299.827	300.614	-0.020	-0.001	-0.000
Max Z	247	9 GENERATE	2.751	6.046	350.745	350.808	0.021	-0.003	0.001
Min Z	246	10 GENERAT	2.749	6.040	-350.742	350.804	-0.021	0.003	0.001
Max rX	127	9 GENERATE	2.923	-13.985	335.523	335.827	0.021	0.003	-0.001
Min rX	126	10 GENERAT	2.923	-13.983	-335.525	335.829	-0.021	-0.003	-0.001
Max rY	46	10 GENERAT	-1.112	-3.075	-21.859	22.102	0.000	0.013	-0.003
Min rY	49	9 GENERATE	-1.112	-3.074	21.858	22.102	-0.000	-0.013	-0.003
Max rZ	29	9 GENERATE	-0.161	3.386	3.909	5.174	0.001	0.007	0.003
Min rZ	17	9 GENERATE	0.162	3.386	3.908	5.174	0.001	-0.007	-0.003
Max Rs	246	9 GENERATE	-3.659	-14.412	350.745	351.060	0.020	-0.003	0.001

Table 12. Displacement result

Maximum displacement of 350mm in the direction on wind due to the load combination 9 (1.7xdead load and 1.7xwind load)

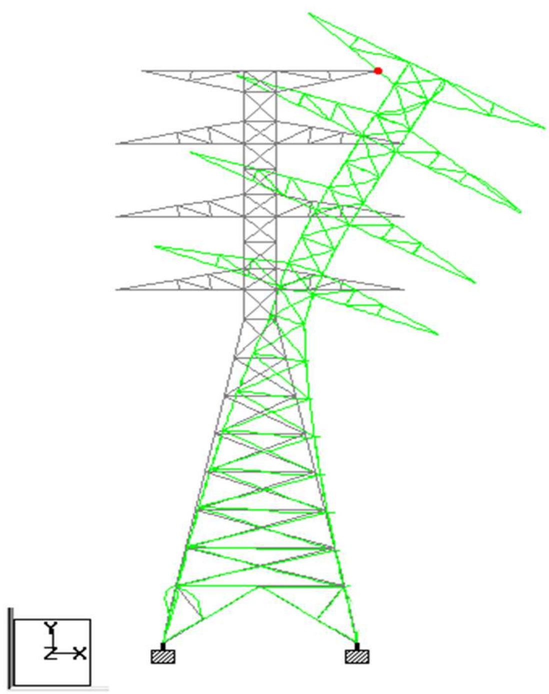


Figure 12. Deflection

Force summary

	Beam	L/C	Node	Fx kN	Fy kN	Fz kN	Mx kNm	My kNm	Mz kNm
Max Fx	26	10 GENERAT	34	879.575	-0.111	-0.512	-0.032	1.557	-0.269
Min Fx	14	9 GENERATE	9	**-780.987**	-2.620	1.964	-0.031	3.460	3.101
Max Fy	284	10 GENERAT	85	-19.761	**14.813**	11.538	0.365	1.210	-0.033
Min Fy	598	9 GENERATE	88	-57.469	**-14.664**	-11.156	-0.365	1.486	-0.071
Max Fz	599	10 GENERAT	92	59.263	-1.596	**12.831**	-0.188	-2.592	-0.260
Min Fz	280	9 GENERATE	89	4.318	1.743	**-13.230**	0.189	-2.041	-0.223
Max Mx	598	10 GENERAT	218	1.483	0.168	1.821	**0.412**	-1.591	-0.081
Min Mx	284	9 GENERATE	85	20.615	-0.012	-2.205	**-0.412**	-1.283	-0.042
Max My	143	9 GENERATE	2	2.723	-0.397	3.872	-0.026	**8.116**	0.362
Min My	133	10 GENERAT	4	2.723	-0.397	-3.872	0.026	**-8.116**	0.362
Max Mz	9	9 GENERATE	18	-629.590	-4.200	1.332	-0.030	0.963	**4.120**
Min Mz	21	10 GENERAT	29	804.159	-3.274	-1.062	-0.030	0.626	**-3.988**

Table 13 . Force summary

Maximum compression force is experienced by beam 284 by load combination (9) and maximum tensile forced is at beam 598 due to load combination (10) i.e. member at the windward side experiences tension and members at the leeward side experiences compression.

Graph

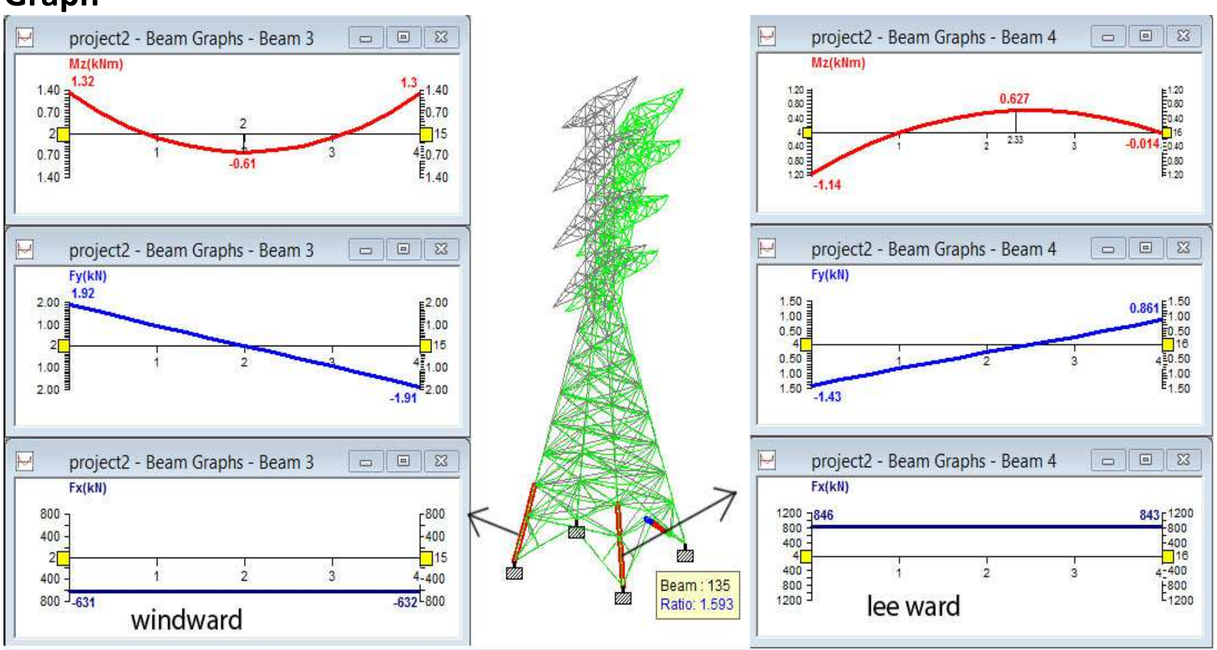


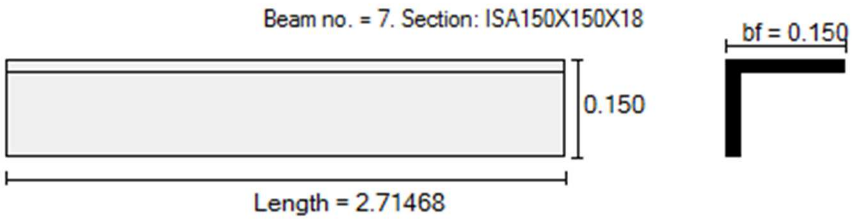
Figure 13. Graphs

Reaction

	Node	L/C	Horizontal Fx kN	Vertical Fy kN	Horizontal Fz kN	Moment Mx kNm	My kNm	Mz kNm
Max Fx	1	10 GENERAT	**184.144**	845.732	230.611	5.210	-8.718	2.124
Min Fx	2	10 GENERAT	**-184.132**	845.748	230.628	5.210	8.718	-2.124
Max Fy	2	10 GENERAT	-184.132	**845.748**	230.628	5.210	8.718	-2.124
Min Fy	1	9 GENERATE	-133.089	**-629.200**	-179.560	-6.233	8.718	-1.101
Max Fz	2	10 GENERAT	-184.132	845.748	**230.628**	5.210	8.718	-2.124
Min Fz	4	9 GENERATE	-184.132	845.748	**-230.630**	-5.211	-8.718	-2.124
Max Mx	4	10 GENERAT	133.078	-629.188	179.572	**6.234**	8.718	1.101
Min Mx	2	9 GENERATE	133.078	-629.187	-179.570	**-6.233**	-8.718	1.101
Max My	3	9 GENERATE	184.144	845.733	-230.613	-5.210	**8.718**	2.124
Min My	3	10 GENERAT	-133.089	-629.200	179.562	6.233	**-8.718**	-1.101
Max Mz	3	9 GENERATE	184.144	845.733	-230.613	-5.210	8.718	**2.124**
Min Mz	4	9 GENERATE	-184.132	845.748	-230.630	-5.211	-8.718	**-2.124**

Table 14. Reaction summary

Design as per IS802



DESIGN STRESSES (NEW, MMS)

YLD	250	FA	182.77
FCZ	0	FTZ	0
FCY	0	FTY	0
FT	36.25	FV	0

Critical load (KN ,METE)

Load	10
Locatio	0
FX	827.765015
MY	0
MZ	0

Code	Result	Ratio	Critical	KLR
IS-802	PASS	0.8917322	COMPRESSION	93.28801

Figure 14. Design as per IS80

Compression force is the most critical at the leg of the tower

Steel takeoff

The following table gives the total amount and type of ISA sections required for the safe and economical transmission tower.

STEEL TAKE-OFF

PROFILE		LENGTH (METE)	WEIGHT (KN)
ST	ISA150X115X15	92.01	26.520
ST	ISA100X100X10	182.53	26.684
ST	ISA90X90X10	270.57	35.397
ST	ISA75X75X10	116.33	12.529
ST	ISA70X70X10	97.83	9.785
ST	ISA110X110X8	83.00	10.852
ST	ISA60X60X10	491.84	41.561
ST	ISA55X55X10	136.87	10.535
		TOTAL =	173.864

Table 15. Steel takeoff as per STAADPro analysis

References

- Jordan Journal of Civil Engineering, Volume 7, No. 4, 2013
- STAAD PRO, Bentley Corporation.
- IS 802 Part 1 Sec 1 1995 Code of practice for use of structural steel in overhead transmission line towers, Part 1
- IS 1893:2000 part 1
- Mohamed Mohsen El-Attar (1998) ”*Non-linear dynamics and seismic response of Power Transmission Lines*”, *PhD Thesis*, McMaster University, Canada